

ADDENDUM

DESCRIPTION:	ADDENDUM NUMBER: ONE
Quarry Lake Redevelopment	DATE OF ISSUE: August 22, 2023
Quarry Lake Redevelopment	ISSUED BY: Amy Bernard
	PAGE(S): 9

INSTRUCTIONS:

- 1. Amend your copy of the proposal in accordance with the detail below.
- 2. Retain one (1) copy for your file; sign and attach to your submission as confirmation that the Addendum was taken into account in your proposal submission.
- 3. Failure to sign and return this form may result in a non-compliant proposal.

DETAILS OF ADDENDUM:

This addendum is issued to include the following:

1. Extension:

The deadline for submissions is extended to September 1 at 2:00pm MT.

2. Clarification:

Delete pages 32-36 from the RFP document.

3. Clarification:

Question: Can you provide some clarification on the foundations required on this project? The tender form and front end of the specifications call for helical screw piles while the Geotechnical report indicates these foundations should be drilled concrete or micropiles. Without any form of borehole data, I am not sure how we could provide a designed solution. Could this be set up as a Cash Allowance? Response: The foundations for the jumping platform are to be grade beams on micropiles, and the staircase and landing is to be supported by helical screw piles as shown on the drawings, designed for the loads shown on plans with review by the geotechnical engineer.

4. Clarification:

Question: Can you provide some clarification on the foundations required on this project? The tender form and front end of the specifications call for helical screw piles while the Geotechnical report indicates these foundations should be drilled concrete or micropiles. Without any form of borehole data, I am not sure how we could provide a designed solution. Could this be set up as a Cash Allowance? Response: The foundations for the jumping platform are to be grade beams on micropiles, and the staircase and landing is to be supported by helical screw piles as shown on the drawings, designed for the loads shown on plans with review by the geotechnical engineer.

5. Clarification:

Question: Is the steel deck part of this scope? If so, are there any details? Response: There is no steel deck part of this scope. Jumping platform is composite deck on weathering steel cantilever structure pre-engineered by supplier to the design loads.

6. Clarification:





Question: Are the micropiles for the deck part of this scope? If so, are there any details? Response: Micropiles for jumping platform to be designed by piling contractor's with engineered shop drawings submitted for review by ISL and geotechnical engineer.

7. Clarification:

Question: There are design parameters listed from the geotech however they also specify they anticipate "shallow" bedrock but I don't see the depth mentioned. Is there a chance you could get that information?

Response: Depth of bedrock at lake shore currently unknown, to be confirmed prior to construction. It is anticipated that bedrock will be encountered in the installation of the foundations.

8. Clarification:

Question: What is the system finish for the steel stair system? Response: Weathering steel. References for primed steel to be removed in Addendum, drawings updated to reflect weathering steel.

9. Clarification:

Question: What is the landing of the steel stair system surface? There is no indication of any form of deck on this landing. I do not see any additional design provided to accept decking. I see a frame with no bracing or stability. Has this stair and platform been designed to meet NBC? I see steel sizes for the framework, stair sections, hinge system and handrails which indicates there is a design. Please confirm who is the designer of the stair system.

Response: Landing revised to composite decking instead of steel grating in this addendum. Landing decking to be Deckoraters Voyage Decking in Khaya or approved alternative as per details revised on Addendum. Steel member sizes and connection details are shown for intent, design of stair is by steel supplier.

10. Clarification:

Question: What is the size and finish of the grating to be used for the stair treads? Why does the stairway steps show grating? Has this stair system been properly designed to meet NBC? 1.5m Clear span stair treads don't appear to be available by Deckorators. Please confirm who is the designer of the stair system.

Response: Stair treads to be Deckoraters Voyage Decking in Khaya or approved alternative as per details revised on Addendum. Stair treads revised to composite decking instead of steel grating in this addendum. Details updated to reflect this change. Design of stair platform steel is by supplier, connection to decking by ISL.

11. Clarification:

Question: What is the platform finish?

Response: Weathering steel structure, with composite decking as shown on plan (Deckoraters Voyage Decking in Khaya or approved alternate).

12. Clarification:

Question: The tender summary states "Supply and install of a prefabricated steel structure jumping platform", however on Detail Plan 1/S03 the note states "Pre-engineered Jumping Platform Structure by others". Which is correct?

Response: Weathering steel cantilever structure to be pre-engineered/fabricated by supplier to the design loads.





13. Clarification:

Question: What are the steel sizes and details for the platform? Response: Weathering steel cantilever structure pre-engineered/fabricated by supplier to the design loads.

14. Clarification:

Question: What is the orientation of the decking? Is it perpendicular, parallel, or diagonal to the main beams?

Response: Decking to be perpendicular to the main beams, with intermediate deck support joists at 300 O/C.

15. Clarification:

Question: On Section 5/S03 the note states "Micropiles by others". Is this correct? Response: "By Others" indicates design by piling contractor, not by ISL. Stamped pile shop drawings to be submitted for structural and geotechnical review.

16. Clarification:

Question: What is the connection detail between the micropiles and the concrete beam? Response: Micropile connections to concrete to be detailed in micropile shop drawings, typically embedded in concrete. See below for example of embedded connection detail from a previous project.







17. Clarification:

Question: What is the connection detail between the concrete beam and the steel jumping platform? Response: Connection to concrete beams to be designed by ISL in conjunction with steel platform shop drawing review.

18. Clarification:

Question: Will other beam solutions be considered such as precast, steel, etc.? Response: Assuming this question is in reference to concrete grade beams, alternate beam solutions to be considered if there is possible cost savings/constructability advantages.

19. Clarification:

Question: In the General Notes on drawing S01 state the piles to be designed for the Factored Loads shown on the plan, which are V=125kN H=25kN & uplift 25kN. The geotech report states the preliminary unfactored loads as 250kN comp., 50kN tension and 50kN lateral. We're the preliminary loads significantly higher than the final loads, as factored loads are typically higher than unfactored loads.

Response: Piles to be designed for factored loads shown on plans in conjunction with geotechnical report, not preliminary geotechnical loads.

20. Clarification:

Question: I do not see the loading for the screw piles supporting the stairs. Response: Pile loads added in addendum drawings.

21. Clarification:

Question: The size of the Chance Helical Anchor of 1.25" seems small. Response: Specified anchors are rated for 178kN compression capacity as per table below. Equivalent alternates will be considered.

Product	Х	XY	Kt	Torque	Ultimate Compression
Series	Shaft Size Across Flats	Diagonal Length	Value	ft-lb (N-m)	Capacity kip (kN)
	Inches (mm)	Inches (mm)			
SS125	1.25" (32)	1.66 (42)	10	4,000 (5 400)	40 (178)
SS5	1.5" (38)	1.91 (49)	10	5,700 (7 730)	57 (254)
SS150	1.5" (38)	1.91 (49)	10	7,000 (9 500)	70 (312)
SS175	1.75" (44)	2.27 (58)	10	10,500 (14 240)	105 (467)
SS200	2.0" (51)	2.57 (65)	10	16,000 (21 700)	160 (712)
SS225	2.25" (57)	2.93 (74)	10	21,000 (28 475)	210 (934)

CHANCE® SQUARE SHAFT (SS) HELICAL PRODUCT RATINGS

22. Clarification:

Question: Can screw piles be used for both the stair and the jumping platform? Response: No, screw piles are not acceptable for jumping platform.

ATTACHMENTS:

Attachment A – Quarry Lake Park Jumping Platform Drawings S01-04 Issued for Addendum #1, dated August 22, 2023.





Name of Firm		
Authorized Signature		
Printed Name	Date	

$\underline{ }$ STRUCTURAL STEEL

- FABRICATE AND ERECT STRUCTURAL STEEL TO CSA S16-14. DESIGN OF CONNECTIONS BY STEEL FABRICATOR UNLESS DETAILED ON THE DRAWINGS. USE MIN. 2 BOLTS PER
- CONNECTION AND DESIGN FOR BEARING CONNECTIONS WITH THREADS INCLUDED IN THE SHEAR PLANE. SPECIALTY STRUCTURAL ENGINEER TO DESIGN CONNECTION FOR FACTORED REACTION SHOWN ON PLAN. WHERE REACTIONS ARE NOT NOTED, DESIGN CONNECTION FOR END REACTION DUE TO A UNIFORMLY DISTRIBUTED LOAD CAUSING A MOMENT EQUAL TO THE MOMENT CAPACITY OF THE BEAM.
- ANCHOR BOLTS TO ASTM A36 OR A307 UNLESS NOTED. OTHERWISE STRUCTURAL BOLTS AND NUTS TO ASTM A325 OR ASTM F3125/F3125M OR ASTM F1554 GRADE 36. TIGHTEN ALL BOLTS WITH AN IMPACT WRENCH.
- PROVIDE A CONTINUOUS 35 MPa GROUT BED BENEATH BASE PLATES AND OTHER CONNECTIONS BEARING ONTO CONCRETE.
- FRAME OPENINGS IN STEEL DECK GREATER THEN 450mm WITH L89x89x6.4 MINIMUM, REFER TO STEEL DECK NOTES AND TYPICAL DETAILS
- SUBMIT SHOP DRAWINGS TO THE ENGINEER AND RECEIVE APPROVAL PRIOR TO FABRICATION. SHOW ALL DETAILS, INCLUDING FIELD WELDS, AND MATERIAL SPECIFICATIONS. SHOP DRAWINGS TO BE SEALED BY A PROFESSIONAL ENGINEER FOR DESIGN OF CONNECTIONS.
- PROVIDE 10mm THICK BEARING STIFFENERS EACH SIDE OF WEB OR TO ONE SIDE OF CHANNEL WEB CENTERED OVER THE SUPPORT WHERE BEAM OR CHANNEL PASSES OVER THE SUPPORT.
- WELD REINFORCEMENT STEEL TO CSA W185. USE WELDABLE REINFORCEMENT TO CSA G30.18 GRADE 400.
- MINIMUM SIZE OF FIELD WELD, 2mm LESS THAN THE THICKNESS OF THE MATERIAL BUT NOT LESS THAN 6mm.
- 10. PROVIDE 6mm CAP PLATES ON ALL HSS SECTIONS UNLESS NOTED OTHERWISE.
- STEEL FABRICATOR SHALL BE CERTIFIED BY CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF CSA W47.1-09, DIVISION 1 OR 2. WELDING TO BE METAL ARC WELDING TO CSA W59-13. PROVIDE COPY OF CERTIFICATE FOR REVIEW BY THE STRUCTURAL ENGINEER.
- AN INDEPENDENT INSPECTION AND TESTING COMPANY WILL BE APPOINTED TO INSPECT STRUCTURAL STEEL IN THE FABRICATION SHOP AND ON SITE. ALLOW FOR A MINIMUM OF ONE VISIT TO SITE AND ONE VISIT TO THE FABRICATION SHOP. PROVIDE REPORT FOR REVIEW BY THE STRUCTURAL ENGINEER.

M M M M M M M M M M M

HELICAL SCREW ANCHOR NOTES

- THE DESIGN, FABRICATION, AND INSTALLATION OF PILES SHALL BE GOVERNED BY THE FOLLOWING CODES:
 - CSA G40.21 STRUCTURAL QUALITY STEELS CSA W48.14 MILD STEEL COVERED ARC-WELDING
 - **FLECTRODES** CSA W48.5 MILD STEEL ELECTRONICS FOR FLUC-CORE ARC-
 - WELDING
 - CSA W47.1-09 CERTIFICATION OF COMPANIES FOR FUSION WELDING OF STEEL STRUCTURES
 - PROVINCIAL OCCUPATIONAL HEALTH AND SAFETY
 - REGULATIONS
- ASTM A252 WELDED AND SEAMLESS STEEL PIPE PILES ASTM A53 WELDED AND SEAMLESS PIPE
- PILES SHALL BE DESIGNED FOR FACTORED LOAD SHOWN ON PLAN.
- PILE WALL THICKNESS AND SIZE SHOWN IS MINIMUM AND SHALL BE INCREASED AS NECESSARY TO ACCOMMODATE INSTALLATION EQUIPMENT.
- SCREW PILES SHALL BE PLACED NOT CLOSER THAN 3 HELIX DIAMETERS FROM ADJACENT PILE U.N.O. PILES SHALL BE PLACED WITHIN 38mm (1 1/2") OF DESIGN DRAWINGS AND SHALL BE PLUMB TO 38mm (1 1/2"). SCREW PILES NOT WITHIN TOLERANCE MAY REQUIRE LOAD TESTING AT STRUCTURAL ENGINEERS DISCRETION.
- SPACING OF HELICES SHALL NOT BE LESS THAN 3 TIMES THE HELIX DIAMETER.
- SPACING OF HELICES SHALL BE A WHOLE NUMBER MULTIPLE OF THE HELIX PITCH.
- SUBMIT SCREW PILE LAYOUT AND SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF JURISDICTION.
- TESTING METHOD TO BE SUBMITTED TO ENGINEER FOR REVIEW. TEST PILES SHALL BE INSTALLED IN THE GENERAL VICINITY OF THE CONSTRUCTION AS APPROVED BY ENGINEER.
- SEE PROJECT GEOTECHNICAL REPORT FOR BOREHOLE LOGS, DESIGN CRITERIA AND METHOD OF CONSTRUCTION.
- SUBMIT LETTER OF COMPLIANCE SEALED BY SCREW PILE ENGINEER AS VERIFICATION THAT PILES HAVE BEEN INSTALLED AS PER DESIGN.

EXCAVATION AND BACKFILL CONT'D

- THE SLAB BASE GRAVEL AND SUBGRADE SOIL MUST BE PROTECTED FROM RAIN, SNOW, EXCESSIVE DRYING AND INGRESS OF FREE WATER DURING AND AFTER THE CONSTRUCTION TO PREVENT ANY FOUNDATION MOVEMENT.
- REFER TO CIVIL DRAWINGS FOR GROUND ELEVATIONS AND 10. DRAINAGE SLOPES.
- CONFIRM EXACT LOCATIONS OF ALL UTILITY LINES WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE COMMENCEMENT OF EXCAVATION.
- 12. IF THE PROJECTED TEMPERATURE AT THE TIME OF FILL PLACEMENT IS BELOW 0° FOR EIGHT HOURS OR MORE. UNIFORMLY GRADED LOW FINES CRUSHED GRANULAR SILL SHALL BE USED. INPARTICULAR, THIS MATERIAL SHALL BE USED WHERE COMPACTION IS REQUIRED IN WINTER CONDITIONS, BUT IS NOT LIMITED TO WINTER USE ONLY.
- THE GRADATION OF THE PROPOSED MATERIAL SHALL BE 13. DEVELOPED BY THE DESIGN ENGINEER FOR THE MECHANICALLY STABILIZED EARTH WALL SYSTEM. ANY CHANGES IN THE DESIGN OF THE REINFORCING STRIPS, FILTER FABRIC, OR OTHER DETAILS SHALL ALSO BE IDENTIFIED AND SUBMITTED TO ISL FOR REVIEW.
- GEOTEXTILE FABRIC SHALL BE PLACED BETWEEN ALL LOW 14. FINES CRUSHED GRANULAR FILL AND ALL OTHER FILL.

CONCRETE FORMWORK

11.

- CONSTRUCT FORMWORK IN ACCORDANCE WITH WCB REGULATIONS AND CSA S269.1-16. FORMWORK DESIGN IS THE RESPONSIBLITY OF THE CONTRACTOR. DO NOT POUR CONCRETE UNTIL FORMWORK HAS BEEN INSPECTED AND CERTIFIED BY THE FORMWORK ENGINEER.
- 2. NO COLUMN OR WALL FORMS SHALL BE REMOVED BEFORE CONCRETE HAS REACHED 75% OF DESIGN STRENGTH OR 4 DAYS, WHICHEVER COMES LATER.
- NO SLAB OR BEAM FORM SHALL BE REMOVED BEFORE CONCRETE HAS REACHED 75% OF DESIGN STRENGTH.
- THE STRENGTH OF CONCRETE IS TO BE DETERMINED BY FIELD CURED CYLINDERS.
- RE-USE FORMWORK AND FALSEWORK SUBJECT TO 5. REQUIREMENTS OF CSA-A23.1/A23.2

CONCRETE REINFORCEMENT NOTES

- TIE ALL BARS SECURELY IN PLACE TO PREVENT DISPLACEMENT. SUPPORT SLAB REINFORCEMENT ON SUITABLE CHAIRS OR SUPPORTS AT MAXIMUM 1.2m CENTRES. PROVIDE CORNER BARS TO MATCH HORIZONTAL WALL REINFORCEMENT.
- CLEAR COVER TO REINFORCEMENT (PRINCIPAL REINFORCEMENT) IS: CAST AGAINST AND PERMANENTLY

EXPOSED TO EARTH	75mm
EXPOSED TO EARTH OR WEATHER	40mm
NOT EXPOSED TO EARTH OR WEATHER	
OR NOT IN CONTACT WITH THE GROUND:	
SLABS AND WALLS (35M OR SMALLER)	20mm
BEAMS AND COLUMNS	40mm

- ALL BOTTOM STEEL TO BE CONTINUOUS AND SPLICED WHERE REQUIRED.
- 4. UNLESS OTHERWISE NOTED, USE CLASS B TENSION SPLICE FOR ALL REINFORCING STEEL.
- 5. UNLESS OTHERWISE NOTED, PROVIDE DOWELS TO MATCH VERTICAL REINFORCING WHEREVER A PILASTER, PIER OR WALL BEGINS.
- 6. PROVIDE MINIMUM 2-15M BARS AROUND ALL OPENINGS LARGER THAN 450mm AT EACH SIDE OF OPENING AND ON DIAGONALS. EXTEND 600mm PAST CORNER. PROVIDE 1-15M 1200mm DIAGONAL AT EACH CORNER OF ALL OPENINGS.
- 7. THE DESIGNATION OF REINFORCEMENT IN DRAWINGS IS AS FOLLOWS: BARS IN TOP OF BEAMS AND SLABS OR IN NEAR FACE OF WALL ARE SHOWN AS A SOLID LINE:
- BARS IN BOTTOM OF BEAMS AND SLABS OR IN FAR FACE OF WALL ARE SHOWN AS A DASHED LINE:
- STRAIGHT BARS: 6-15M4500 MEANS 6-15M BARS, 4500 LONG BENT BARS:
- THE BAR LENGTHS NOTED ARE EXCLUSIVE OF THE STANDARD HOOK.

STRUCTURAL FIELD REVIEW NOTES

- THE CONTRACTOR SHALL COOPERATE WITH ALL TESTING, INSPECTION AND QUALITY CONTROL PERSONNEL REQUIRED ON THE SITE AND WILL PROVIDE CASUAL LABOUR FORCES AS REQUIRED TO ASSIST IN ALL THE FIELD **REVIEW PROCEDURES. THE CONTRACTOR SHALL GIVE** REASONABLE NOTICE TO THESE AGENCIES PRIOR TO REQUIRING THEIR SERVICES.
- 2. ALL REINFORCEMENT SHALL BE REVIEWED IN PLACE PRIOR TO PLACING THE CONCRETE BY ISL ENGINEERING & LAND SERVICES LTD. ALL REINFORCEMENT SHALL BE IN PLACE AND SECURED AT THE TIME OF THE REVIEW. PROVIDE MINIMUM 48 HOURS NOTICE PRIOR TO POURS.
- REFER TO GENERAL NOTES AND MATERIAL SPECIFICATION NOTES FOR ADDITIONAL REQUIREMENTS.

MATERIAL SPECIFICATIONS CONT'D

ELEMENT	COMPRESSIVE STRENGTH (MPa) 28 DAYS	EXPOSURE CLASS	SPECIAL REQUIREMENTS & REMARKS
FOOTINGS, GRADE BEAMS	30	N/F-2	

- WHERE EXPOSURE CLASS IS NOTED "N / F2", USE F-2 EXPOSURE CLASS FOR PERIMETER AND EXTERIOR ELEMENTS ABOVE THE FROST LINE, AND ELEMENTS IN INTERIOR UNHEATED SPACES, THAT ARE SUSCEPTIBLE TO FREEZING. USE N EXPOSURE CLASS FOR ELEMENTS PROTECTED FROM FREEZING.
- LIMIT MAXIMUM AGGREGATE SIZE TO 10mm FOR COLUMNS WITH SMALLEST DIMENSION LESS THAN 300mm, FOR WALLS LESS THAN 200mm THICK, AND FOR TOPPINGS LESS THAN 75mm THICK.
- DO NOT USE ADMIXTURES OTHER THAN AIR ENTRAINMENT STANDARD WATER REDUCERS OR SUPER PLASTICIZERS WITHOUT PRIOR APPROVAL OF ENGINEER. CALCIUM CHLORIDE ADMIXTURES ARE NOT PERMITTED UNLESS NOTED.
- PROVIDE AIR ENTRAINMENT IN ACCORDANCE WITH CSA A23.1 FOR ALL EXTERIOR CONCRETE.
- REJECT ALL CONCRETE WHEN TIME BETWEEN BATCHING AND PLACING EXCEEDS 2 HOURS.
- DO NOT ADD WATER TO CONCRETE ON SITE UNLESS 8. AUTHORIZED BY ENGINEER.
- CONSOLIDATE ALL CONCRETE USING MECHANICAL 9. VIBRATORS.

CONCRETE REINFORCEMENT

- PROVIDE REINFORCEMENT TO CAN/CSA-A23.3 AND CSA 1. G30.18 AS FOLLOWS:
 - 300 MPa FOR 10M OR SMALLER 400 MPa FOR 15M OR LARGER
- PROVIDE NEW DEFORMED BARS TO CSA G30.18 GRADE 400.

EXCAVATION AND BACKFILL

- THE OWNER SHALL OBTAIN THE SERVICES OF A QUALIFIED TESTING AGENCY TO PERFORM COMPACTION TESTS AS REQUESTED BY THE ENGINEER.
- EXCAVATE TO THE LEVELS NOTED ON THE DRAWINGS FOR THE EXTENT OF THE STRUCTURE. STRIP THE OVER-EXCAVATED AREA OF ALL SILT. FOOTING LEVELS SHOWN ARE NOT FINAL AND MAY VARY ACCORDING TO SITE CONDITIONS. EXTEND ALL FOOTINGS TO A BEARING LAYER APPROVED BY THE GEOTECHNICAL ENGINEER.
- FILL TO THE DESIGN SUBGRADE WITH PITRUN GRAVEL COMPACTED TO 98% STANDARD PROCTOR DENSITY
- KEEP EXCAVATION FREE OF WATER WHILE FILL AND CONCRETE FOUNDATION IS PLACED.
- PROTECT BOTTOM OF EXCAVATION FROM FROST. DO NOT PLACE 5. CONCRETE ON FROZEN SOIL.
- WHERE POSSIBLE, BACKFILL WALLS FROM BOTH SIDES SIMULTANEOUSLY TO EQUALIZE SOIL PRESSURE. WALL RESISTING LATERAL EARTH PRESSURES ARE NOT TO BE BACKFILLED UNTIL THE BASEMENT AND MAIN FLOOR SLABS ARE IN PLACE.
- REMOVE ALL VEGETATION. ORGANIC SOIL AND CONSTRUCTION DEBRIS FROM BUILDING AND CONSTRUCTION AREA TO EXPOSE I NORGANIC SUBGRADE SOIL. THE EXPOSED EXCAVATION MUST BE INSPECTED FOR APPROVAL PRIOR TO PROOF ROLLING. PROOFROLL THE EXPOSED INORGANIC SUBGRADE SOIL TO PROVIDE A GOOD BASE FOR COMPACTING THE FIRST LIFT OF MATERIAL TO THE SPECIFIED DENSITY.
- ANY SOFT SUBGRADE SOIL ENCOUNTERED SHOULD BE SUB-EXCAVATED AND REPLACED WITH FREE DRAINING PITRUN GRAVEL. SOFT SUBGRADE SOIL WILL LIKELY BE ENCOUNTERED DURING SITE PREPARATION. COMPACT FREE DRAINING PITRUN GRAVEL TO NOT LESS THAN 95% STANDARD PROCTOR DRY DENSITY.
 - A MINIMUM OF 200 [8"] THICK NON-PLASTIC CRUSHED GRAVEL MUST BE PLACED BENEATH THE ENTIRE SLAB AND ABOVE THE PREPARED SUBGRADE SOIL. THE CRUSHED GRAVEL MUST BE UNIFORMLY COMPACTED TO 95% STANDARD PROCTOR DRY DENSITY. THE GRAVEL SHALL MEET THE FOLLOWING GRADATION STANDARD:

SIEVE SIZE	% PASSING BY WEIGH
19mm [3/4]	100
12.5mm [1/2"]	70-100
4.75mm [3/16"]	40-60
1.18mm [0.0469"]	25-45
0.30mm [0.0117"]	10-25
0.075mm [0.0029"]	2-12

DESIGN	DATA	
DESIGN COD	E : NATIONAL BUILDING CODE - 2019 A E CATEGORY: NORMAL Is = 1.0, Iw = 1.0	LBERTA EDITION), IE = 1.0
ROOF DEAD LOADS JUMP	: ING PLATFORM	——— 1.27 kPa
ENVIRONMEN GROU RAIN DESIG	NTAL LOADS: JND SNOW LOAD (Ss)	———— 3.20 kPa ———— 0.10 kPa ———— 2.66 kPa
<u>FLOOR</u> LIVE LOADS:		
DECK		4.80 kPa
WIND	LOADS: HOURLY WIND PRESSURE (1/	50) —— 0.37 kPa
SEISM	AIC DATA: $Sa(0.2) = 0.278$ $Sa(0.5) = 0.183$ $Sa(1.0) = 0.098$ $Sa(2.0) = 0.046$ $Sa(5.0) = 0.016$ $Sa(10.0) = 0.000$ PGA (g) = 0.12 PGV (m/s) = 0.1 SITE CLASS: E	3 3 3 3 5 3 5 3 5 3 8 097 5
SOIL CONDIT	IONS:	
THE FOUNDA INVESTIGATI BY WATT CO	ATION HAS BEEN DESIGNED BASED ON ON REPORT #3914.G01 DATED JANUAF NSULTING GROUP.	I THE GEOTECHNICAL RY 10, 2022 PREPARED
UNHEATED F ULTIMATE BE ULTIMATE GF	ROST DEPTH EARING CAPACITY	————2.0 m ———— 200 kPa ———— 150 kPa
MATERI	AL SPECIFICATIONS	
<u>STEEL</u>		
1.	PROVIDE STRUCTURAL STEEL TO C G40.20-13/G40.21-13 OR ASTM STAND FOLLOWING SHOULD HAVE GRADES • STRUCTURAL SHAPE • HOLLOW STRUCTURA CLASS 'C' • COLUMN BASE PLATE • MISCELLANEOUS PLA • PIPE SECTIONS	SA/CAN- DARD A 992/A992M. THE MINIMUM OF: S350W AL SECTIONS350W ES300W ITES260W ASTM A53, 241W
<u>WELDING</u>		
1.	WELDING TO BE METAL ARC WELDIN WELDERS APPROVED BY THE CANA TO REQUIREMENTS OF CSA W47.1. (MADE AVAILABLE UPON REQUEST.	NG TO CSA W59 BY DIAN WELDING BUREAU CERTIFICATE TO BE
2.	WELD REINFORCEMENT STEEL TO C	CSA W186.
1. CONCRETE	 TO CONFORM TO THE FOLLOWING U BOLTS: ASTM A307, A325, A32 F182. THREADED ROD: ASTM A307 WASHERS: MALLEABLE C STEEL ROD: ASTM A449 OR A 	J.N.O.: 25M, A490, A490M OR AST IRON ASTM A307
1.	CAST-IN-PLACE CONCRETE AND CO	NSTITUENT MATERIAL
2.	CONCRETE SHALL BE PROPORTION	ED AND PRODUCED IN

CONCRETE SHALL BE MIXED, PLACED AND CURED IN ACCORDANCE WITH CSA A23.1.

ABBREVIATIONS

ALT. ARCH. BLL BUL BM. BOT. BTWN CL. CLR. COL. CONC. CONT.	ALTERNATE ARCHITECTURAL BOTTOM LOWER LAYER BOTTOM UPPER LAYER BEAM BOTTOM BETWEEN CENTER LINE CLEAR COLUMN CONCRETE CONTINUOUS	DWG. DWLS E.E. ELF. ELEC. EL. ELEV. E.S. E.W. EXIST. EXT. F.D.	DRAWING DOWELS EACH END EACH FACE ELECTRICAL ELEVATION ELEVATION EACH SIDE EACH WAY EXISTING EXTERIOR FLOOR DRAIN
CONT.		F.D.	
U.P.	PENETRATION	GALV.	GALVANIZED
C/W	COMPLETE WITH	G.L.	GRID LINE
D.L. DP.	DEAD LOAD DEEP	H.D.G. HORIZ.	HOT DIPPED GALVANIZED HORIZONTAL

GENERAL

ALL PLAN DIMENSIONS ARE MEASURED TO: FACE OF WALL

- CENTERLINE ROUGH OPENING OR OTHERWISE NOTED. CONTACT ISL ENGINEERING FOR VERIFICATION IF REQUIRED
- LOCATE DOOR FRAMES 100mm AWAY FROM NEAREST WALL UNLESS NOTED OTHERWISE.
- SEE CIVIL, LANDSCAPE, MECHANICAL AND ELECTRICAL DRAWINGS FOR EXTENT AND LOCATIONS OF RELATED FIXTURES OR PENETRATIONS.
- CONFIRM ROUGH OPENING SIZES WITH SUPPLIERS AND PROVIDE FURRING AS REQUIRED.
- 5. THESE DRAWINGS ARE REPRESENTATIVE OF THE WORK. CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL MEANS AND METHODS TO CONSTRUCT THE WORK REPRESENTED HEREIN.
- 6. CONTRACTOR MUST EXECUTE ALL WORK IN ACCORDANCE WITH THE MOST CURRENT APPLICABLE PROVINCIAL, NATIONAL AND MUNICIPAL BUILDING CODES, FIRE CODES AND STANDARDS SPECIFIED.
- 7. ALL MATERIALS AND INSTALLATIONS SHALL BE GUARANTEED FOR A PERIOD OF AT LEAST ONE YEAR FROM THE DATE OF THE CONSTRUCTION COMPLETION CERTIFICATE.
- 8. THE CONTRACTOR SHALL EXAMINE ALL CONTRACT DOCUMENTS, CHECK DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER FOR CLARIFICATION PRIOR TO COMMENCING CONSTRUCTION. DISCREPANCIES NOT REPORTED ARE THE RESPONSIBILITY OF THE CONTRACTOR. CHECK AND VERIFY ALL DIMENSIONS WITH THE CONSULTANT DRAWINGS BEFORE COMMENCING WITH ANY WORK. NOTIFY THE ARCHITECT OF ANY ERRORS OR OMISSIONS.
- 9. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE CIVIL, LANDSCAPE, MECHANICAL, AND ELECTRICAL DRAWINGS.
- 10. TEMPORARY SUPPORT AND TEMPORARY AND PERMANENT BRACING OF LOAD BEARING AND NON-LOAD BEARING ELEMENTS DURING CONSTRUCTION TO RESIST DEAD, LIVE AND CONSTRUCTION LOADS IS THE RESPONSIBILITY OF THE CONTRACTOR. DESIGN OF THE TEMPORARY SUPPORTS IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 11. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
- 12. THE GENERAL CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR REVIEW BY THE ENGINEER OF RECORD. ALLOW ADEQUATE TIME FOR REVIEW BY THE ENGINEER OF RECORD PRIOR TO FABRICATION OR ERECTION. SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO
- SUBMISSION FOR REVIEW BY THE ENGINEER OF RECORD. SHOP DRAWINGS SHALL BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT PROVINCE OR STATE OF JURISDICTION. THE SHOP DRAWING ENGINEER SHALL ENSURE THAT THE FABRICATION AND ERECTION OF THESE ELEMENTS ARE IN ACCORDANCE WITH THEIR DESIGN AND THAT THE DESIGN IS IN ACCORDANCE WITH ALL RELEVANT CODES AND REGULATIONS. SHOP DRAWINGS SHALL BE SUPPLIED FOR REVIEW FOR THE
- FOLLOWING BUT NOT LIMITED TO: A. REINFORCING STEEL FOR CONCRETE
- B. STRUCTURAL STEEL SHAPES AND PLATES
- C. STEEL DECKING

I.F.

INT.

LG.

L.L.

LLH

- D. WELDED ASSEMBLIES E. ARCHITECTURAL ASSEMBLIES
- 13. ALL DESIGN TO CONFORM TO THE NBC-2019 AE, AND ALL OTHER APPLICABLE CODES AND PRACTICES AND BEST PRACTICES.
- 14. FIELD REVIEWS:
- NOTIFY THE ENGINEER 48 HOURS IN ADVANCE FOR FIELD **REVIEWS AND APPROVAL OF THE FOLLOWING:**
- A. CONCRETE REINFORCEMENT BEFORE EACH CONCRETE POUR
- B. STRUCTURAL STEEL BEFORE COVERING UP
- 15. THE DESIGN HAS BEEN PREPARED BASED ON THE ASSUMPTION THAT THE OWNER AND/OR OPERATOR HAS A SITE SAFETY PLAN IN PLACE TO ADDRESS AND MITIGATE SAFETY HAZARDS. BOTH COMMON AND SPECIFIC TO THIS PROJECT.
- 16. TYPICAL DETAILS AND GENERAL NOTES APPLY UNLESS NOTED OTHERWISE ON PLANS.
- 17. GENERAL CONTRACTOR TO ADVISE AND COORDINATE WITH CONSULTANTS IF CONFLICTS ARISE BETWEEN SPECIFICATIONS AND DRAWINGS PRIOR TO PROCEEDING WITH SHOP DRAWINGS, FABRICATION, AND/OR CONSTRUCTION.
- INSIDE FACE SIM. SIMILAR INTERIOR S.O.G. SLAB ON GRADE LONG STIR. STIRRUP THK. THICK LIVE LOAD LONG LEG HORIZONTAL THRU THROUGH LLV LONG LEG VERTICAL TLL TOP LOWER LAYER MAX. MAXIMUM T.O. TOP OF MECH. MECHANICAL T.O.S. TOP OF STEEL/SLAB MIN. MINIMUM TUL TOP UPPER LAYER TYP. TYPICAL N.T.S. NOT TO SCALE O/C ON CENTRE T & B TOP AND BOTTOM O.F. OUTSIDE FACE U.N.O. UNLESS NOTED OPP. OPPOSITE OTHERWISE R.D. ROOF DRAIN U/S UNDERSIDE **REINF. REINFORCING** VERT. VERTICAL R/W REINFORCED WITH WT. WALL THICKNESS S.D.L. SUPERIMPOSED DEAD LOAD



Calgary Office 403.254.0544 4015 7 St SE Calgary, AB T2G 2Y9

This drawing, as an instrument of service, is the property of the Engineer and may not be reproduced without permission and unless the reproduction carries their name. All designs and other information shown on this drawing are for the use on the specified project only and shall not be used otherwise without the written permission of the

Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions shown on the job and the Engineer shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to the Engineer for approval prior to preceeding with fabrication.

Stamp/Seal

Revisions/Submissions

-	No.	DATE	DESCRIPTION
-	1	2023.05.26	ISSUED FOR TENDER
	2	2023.08.22	ADDENDUM #1

Submission Phase/Status

TENDER

Project

QUARRY LAKE PARK **JUMPING PLATFORM**

Owner/Client

TOWN OF CANMORE

Title

GENERAL NOTES

Scale 1:1 Drawn/Designed/Design Checked: Date LY /LH /RC 2022-01-07 **S01** 61943

Sheet

Project No:



Stamp	o/Seal		
Revisi	ions/Submis	ssions	
No. 1	DATE 2023.05.2	DESCRI 26 ISSUED FOR TE	PTION ENDER
Subn TE Proje	nission Pha	se/Status	
Subn TE Proje QL JU	nission Pha ENDEI ect JARR IMPIN	se/Status R Y LAKE I IG PLATF	PARK Form
Subn TE Proje QL JU	nission Pha ENDEI Oct JARR JMPIN er/Client	se/Status R Y LAKE I IG PLATF	PARK FORM
Subn TE Proje QL JU	nission Pha ENDEI ect JARR JMPIN er/Client DWN (TE P	se/Status R Y LAKE I IG PLATE	PARK
Subn TE Proje QL JU	nission Pha ENDEI ect JARR JMPIN er/Client DWN (TE P	se/Status R Y LAKE I IG PLATF	PARK ORM
Subn TE Proje QL JU	nission Pha ENDEI ect JARR JMPIN er/Client DWN (TE P	se/Status R Y LAKE I IG PLATE OF CANM LAN	PARK ORM
Subn TE Proje QL JU Owne TC Title SCale Drawr LY	nission Pha ENDEI ect JARR	se/Status R Y LAKE I G PLATE OF CANN LAN /Design Checked:	PARK ORM
Subn TE Proje QL JU Owne TC Title SCale Drawr LY	nission Pha ENDEI act JARR	se/Status R YLAKE I GPLATE OF CANN LAN /Design Checked:	PARK ORM

Calgary Office 403.254.0544 4015 7 St SE Calgary, AB T2G 2Y9

Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions shown on the job and the Engineer shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to the Engineer for approval prior to preceeding with fabrication.

Note This drawing, as an instrument of service, is the property of the Engineer and may not be reproduced without permission and unless the reproduction carries their name. All designs and other information shown on this drawing are for the use on the specified project only and shall not be used otherwise without the written permission of the Engineer.



MARK SIZE COMMENTS	
BM1 600Wx600DP R/W 4-15M TOP & BOT. W/ 10M STIRRUPS @ 30)0 O/C

